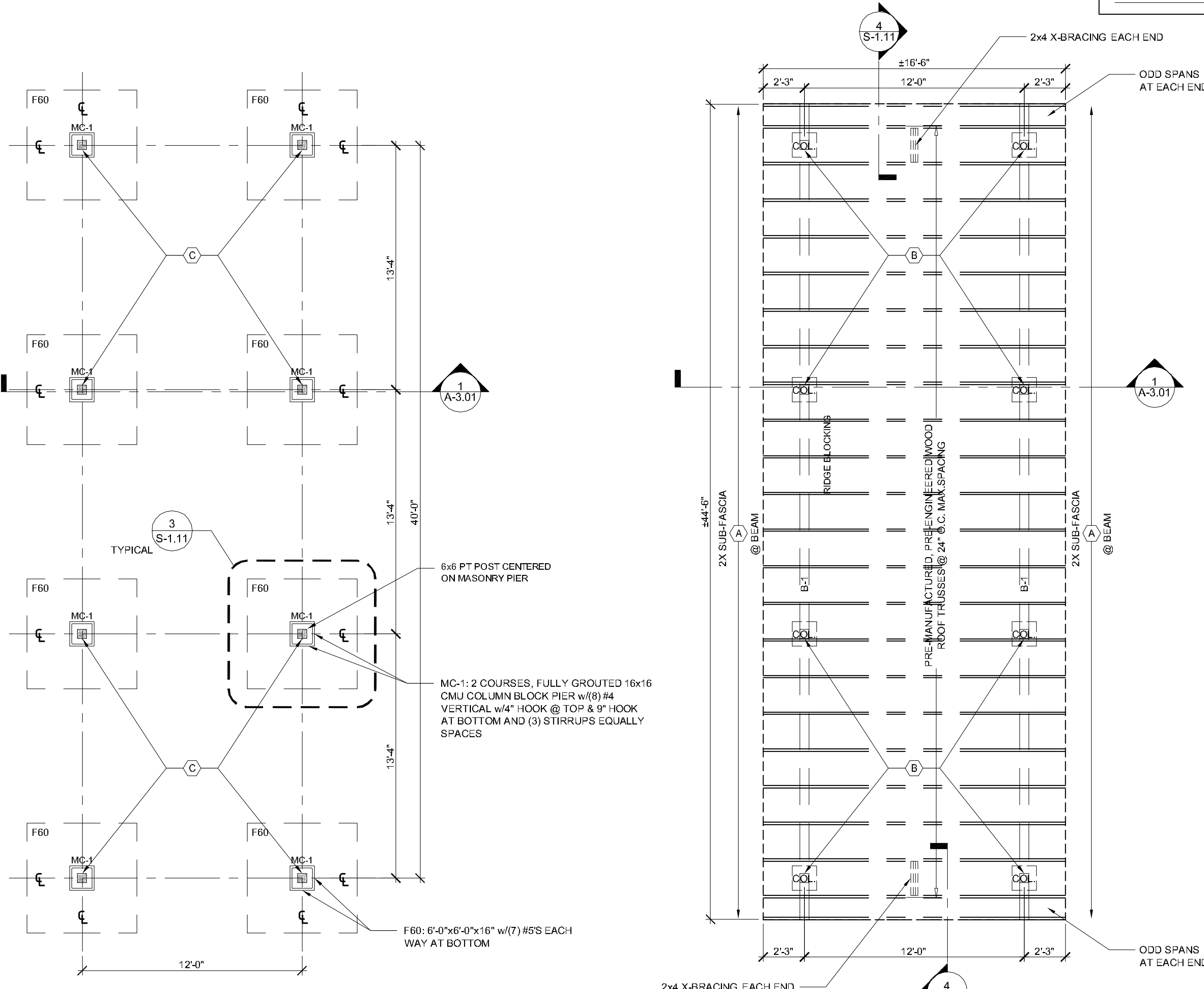


BID ALTERNATE #1

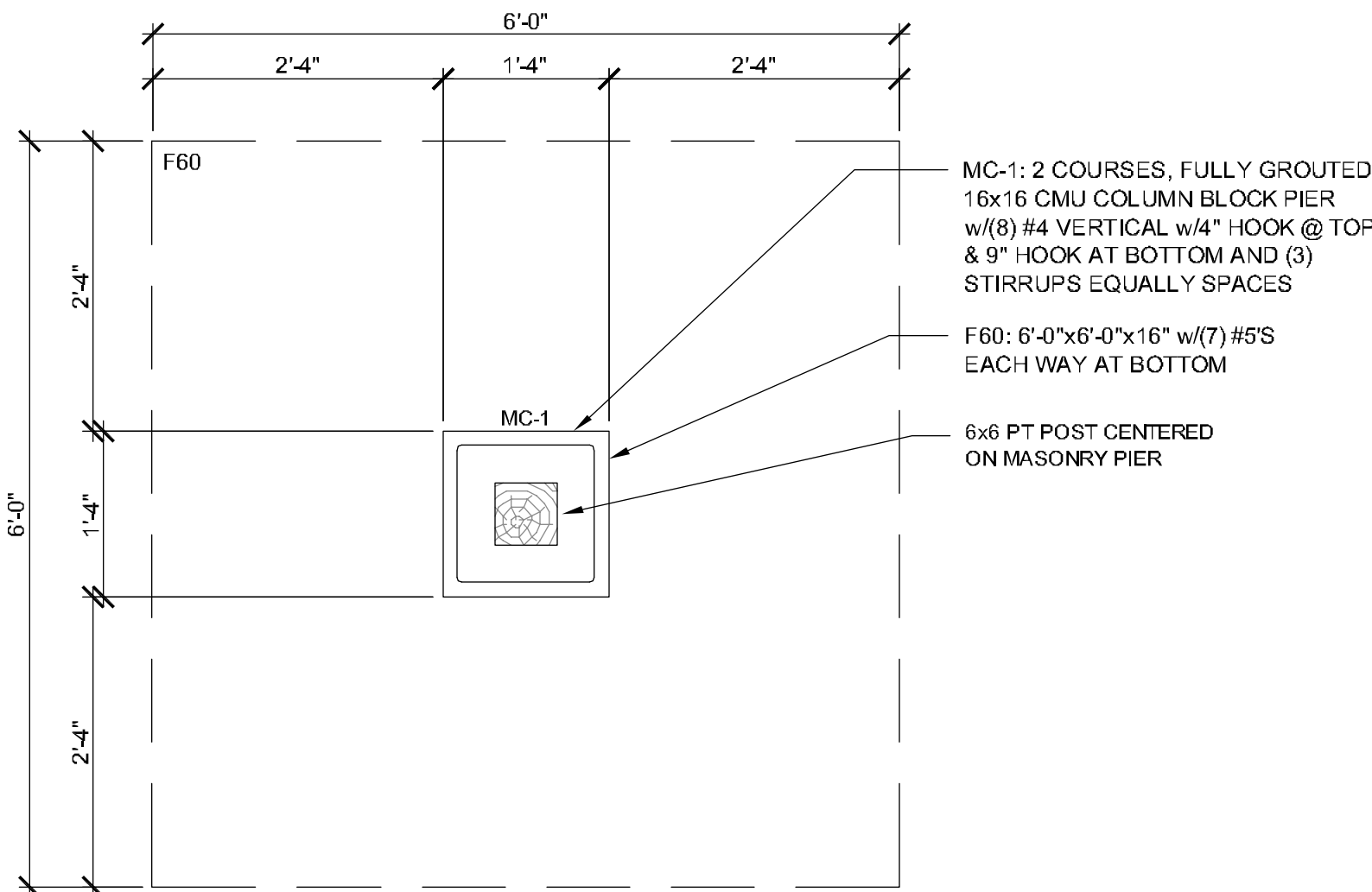


CANOPY FOUNDATION PLAN

SCALE: 1/4" = 1'-0"

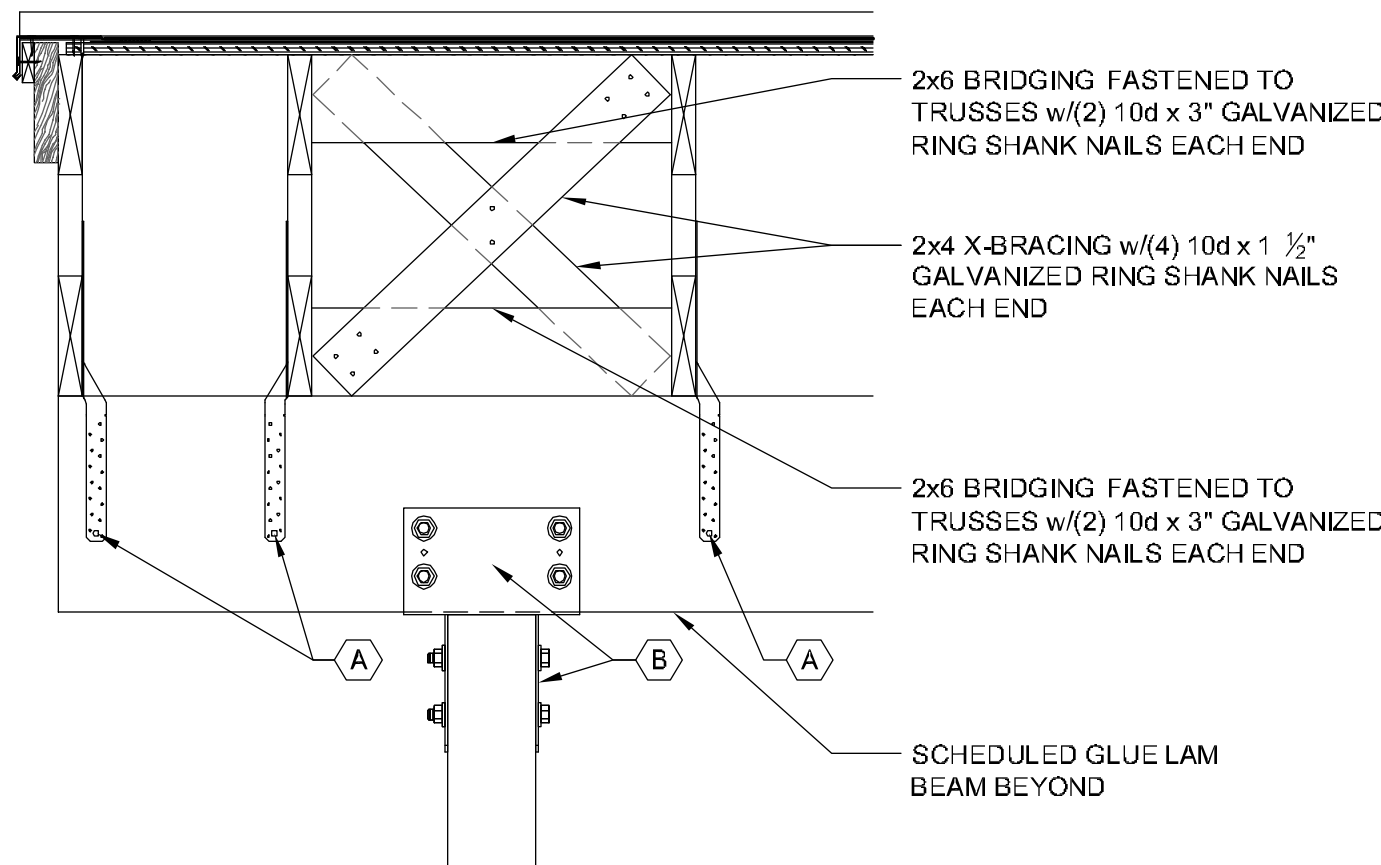
CANOPY ROOF FRAMING PLAN

SCALE: 1/4" = 1'-0"



ENLARGED PLAN DETAIL @ TYPICAL COLUMN BASE/FOOTING

SCALE: 3/4" = 1'-0"



ENLARGED BRIDGING DETAIL @ TYPICAL END COLUMN

SCALE: 1" = 1'-0"

STRUCTURAL NOTES

CONTRACTOR NOTE:

THE CONTRACTOR IS SOLELY RESPONSIBLE FOR INITIATING, MAINTAINING AND SUPERVISING ALL SAFETY PRECAUTIONS AND PROGRAMS IN CONNECTION WITH THE WORK. BRENT A. WOOD ARCHITECTURE LLC, IS NOT RESPONSIBLE FOR THE MEANS AND METHODS OF CONSTRUCTION OR FOR RELATED SAFETY PRECAUTIONS AND PROGRAMS.

CODES AND STANDARDS

- WIND LOADS ARE PER ASCE 7-10 MINIMUM DESIGN LOADS FOR BUILDINGS AND OTHER STRUCTURES, FOR A 170 MPH U.L.T. WIND SPEED (132 MPH NOMINAL WIND SPEED), EXPOSURE D3, ENCLOSED BUILDING (+/-0.18 INTERNAL PRESSURE COEFFICIENT) AND RISK CATEGORY II
- THE PROJECT WAS DESIGNED IN ACCORDANCE WITH THE:
 - FLORIDA BUILDING CODE 6TH EDITION (2017)
 - BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE (ACI 318)
 - SPECIFICATION FOR STRUCTURAL CONCRETE FOR BUILDINGS (ACI 301)
 - MANUAL OF STD. PRACTICE FOR DETAILING REINFORCED CONCRETE STRUCTURES (ACI 315)
 - NATIONAL DESIGN SPECIFICATION, WOOD CONSTRUCTION NDS/CURRENT EDITION
 - BUILDING CODE REQUIREMENTS AND SPECIFICATIONS FOR MASONRY STRUCTURES (ACI 530, 530.1/ASCE 5, 6/TMS 402, 602)
 - ASCE 7-11
- BEFORE ORDERING ANY MATERIALS OR DOING ANY WORK, THE CONTRACTOR SHALL VERIFY ALL MEASUREMENTS TO PROPERLY SIZE OR FIT THE WORK. NO EXTRA CHARGE OR COMPENSATION WILL BE ALLOWED BY THE OWNER RESULTING FROM THE CONTRACTOR'S FAILURE TO COMPLY WITH THIS REQUIREMENT.
- DISCREPANCIES SHALL BE BROUGHT TO THE ATTENTION OF THE ARCHITECT AND ENGINEER BEFORE PROCEEDING WITH ANY WORK.
- ALL DIMENSIONS AND ELEVATIONS SHOWN ON THE STRUCTURAL DRAWINGS MUST BE VERIFIED AND COORDINATED WITH THE ARCHITECTURAL DRAWINGS BY THE CONTRACTOR BEFORE PROCEEDING WITH THE CONSTRUCTION. DISCREPANCIES SHALL BE BROUGHT TO THE ATTENTION OF THE ARCHITECT OR ENGINEER IN WRITING BEFORE PROCEEDING WITH ANY WORK.

SPECIALTY ENGINEERED PRODUCTS

- THE GENERAL CONTRACTOR IS RESPONSIBLE TO COORDINATE THE PROPER SUBMISSION OF SPECIALTY ENGINEERED SHOP DRAWINGS WHICH SHALL BE SIGNED AND SEALED BY AN ENGINEER REGISTERED IN THE STATE OF FLORIDA. IT IS THE GENERAL CONTRACTOR'S RESPONSIBILITY TO ASSURE THAT THE SPECIALTY ENGINEERED SHOP DRAWINGS ARE SUBMITTED IN A TIMELY MANNER SO AS TO ALLOW REVIEWS AND RESUBMISSIONS AS REQUIRED. ALL SPECIALTY ENGINEERED PRODUCTS SHALL BE DESIGNED FOR THE APPROPRIATE GRAVITY LOADS AND WIND LOADS INCLUDING UPLIFT AND LATERAL LOADS. INTERIOR SPECIALTY PRODUCTS SHALL BE DESIGNED FOR LATERAL LOADS TO ASSURE STABILITY

FOUNDATION

- ALL SITE PREPARATION AND EXCAVATION WORK IS TO BE PERFORMED IN STRICT ACCORDANCE WITH THE RECOMMENDATIONS ON SOILS AND FOUNDATIONS INVESTIGATION PREPARED BY AN APPROVED TESTING LABORATORY PRIOR TO FOUNDATION WORK.
- BOTTOM OF FOOTINGS ASSUMED TO BEAR ON SOIL CAPABLE OF SAFELY SUPPORTING 2500 PSF
- SOILS SUPPORTING ALL FOOTINGS MUST BE INSPECTED AND APPROVED BY A REGISTERED SOILS ENGINEER BEFORE COMMENCING WORK. APPROVAL IN WRITING MUST INDICATE THE SOIL IS ADEQUATE TO SAFELY SUSTAIN SPECIFIED SOIL BEARING PRESSURE.
- BOTTOM OF ALL EXTERIOR MONO-FOOTINGS SHALL BE MINIMUM 12 INCHES BELOW EXTERIOR FINISH GRADE

CONCRETE

- ALL CONCRETE SHALL BE READY MIX AND MEET THE FOLLOWING REQUIREMENTS:
 - A MINIMUM COMPRESSIVE STRENGTH OF 3000 PSI @ 28 DAYS
 - SUMPS SHALL BE 6 INCHES +/- 1 INCH
 - CONCRETE SHALL HAVE 3.5% +/- 1.5% ENTRAPPED AIR
 - ALL CONCRETE TO HAVE MAXIMUM WATER/CEMENT RATIO OF 0.55
 - JOBSITE WATER SHALL NOT BE ADDED.
- CONCRETE COVER FOR REINFORCING STEEL SHALL BE AS REQUIRED BY ACI SPECIFICATIONS.
- LAP ALL BARS PER ACI MINIMUM REQUIREMENTS FOR TENSION LAP SPLICE BUT NOT LESS THAN 48 BAR DIAMETERS. LAP ALL WWF A MINIMUM OF 12 INCHES
- ALL HOOKS SHOWN IN STEEL REINFORCING BARS SHALL BE PER ACI RECOMMENDATIONS (NOT LESS THAN 12 BAR DIAMETERS PAST BEND) UNO
- WELDED WIRE FABRIC SHALL COMPLY WITH ASTM A 185, UNLESS OTHERWISE SPECIFIED. PLACE FABRIC 2" CLEAR FROM TOP OF THE SLAB IN SLAB ON GRADE AND SUPPORT ON SLAB BOLSTERS SPACED AT 3'-0" O.C.
- ALL REINFORCING STEEL SHALL BE MANUFACTURED FROM HIGH STRENGTH BILLET STEEL CONFORMING TO ASTM DESIGNATION A 615 GRADE 60.
- AT CORNERS OF CONCRETE BEAMS, TIE BEAMS, BOND BEAMS AND CONTINUOUS WALL FOOTINGS, PROVIDE (1) #5 X 5'-0" BENT BAR FOR EACH HORIZONTAL BAR SCHEDULED AT EACH FACE.

MASONRY

- MASONRY UNITS SHALL BE, LOAD BEARING ASTM C90, NORMAL WEIGHT AND SHALL BE LAID IN A FULL BED OF MORTAR IN RUNNING BOND
- THE COMPRESSIVE STRENGTH OF MASONRY (F_m) SHALL BE 2,500 PSI AS CALCULATED IN ACCORDANCE WITH ASTM C1314 (HIGH STRENGTH)
- ALL MORTAR SHALL BE TYPE M ONLY, IN ACCORDANCE WITH ASTM SPECIFICATION C270
- GROUT SHALL BE A HIGH SLUMP MIX IN ACCORDANCE WITH ASTM SPECIFICATION C476 AND HAVING A MINIMUM COMPRESSIVE STRENGTH OF 3000 PSI

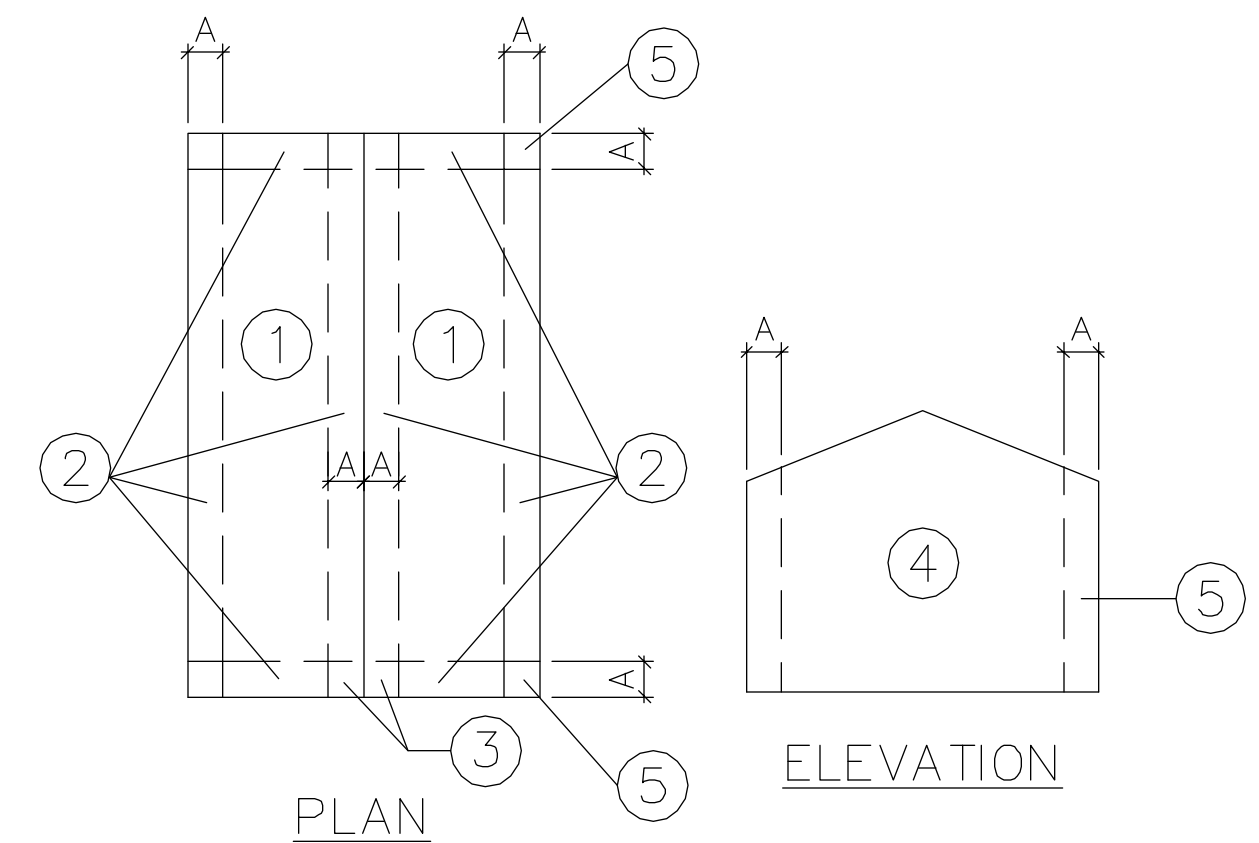
WOOD

- ALL STRUCTURAL WOOD MEMBERS ARE DESIGNED AS "DRY-USE". MOISTURE CONTENT MUST BE 19% OR LESS. STORE WOOD FRAMING ABOVE GROUND AND UNDER TARPS WITH PROPER AIR CIRCULATION.
- ALL LUMBER SHALL BE SOUTHERN PINE SPECIES #2 GRADE OR APPROVED EQUAL. ALLOWABLE DESIGN STRESSES SHALL FOLLOW NATIONAL DESIGN SPECIFICATION (NDS) (LATEST EDITION).
- PROVIDE SP ACQ PRESURE TREATED LUMBER IN ACCORDANCE WITH AWPA STANDARDS TO A MINIMUM 0.40 PCF RETENTION WHERE LUMBER IS IN CONTACT WITH CONCRETE/MASONRY OR OUTSIDE OF BUILDING. ALL METAL CONNECTORS IN CONTACT WITH PRESURE TREATED LUMBER SHALL BE GALVANIZED WITH A RATING OF G-185 AND CONFORM TO ASTM A663. ALL NAILS AND SCREWS USED WITH PRESURE TREATED LUMBER ARE TO BE HOT-DIPPED GALVANIZED AND TO CONFORM TO ASTM A153 CLASS D. ELECTROGALVANIZED FASTENERS SHALL HAVE A CLASS RATING PER ASTM B695 NO LESS THAN 55. ALUMINUM NOT TO BE USED IN DIRECT CONTACT WITH ACQ TREATED LUMBER.

WIND LOAD SCHEDULE (NOMINAL)

COMPONENTS AND CLADDING	ROOF WIND LOADS			WALL WIND LOADS (SEE NOTE 1)	
	ROOF AREA			WALL AREA	
	1	2	3	4	5
PRESSURE (PSF)	+43.3	+43.3	+43.3	+47.3	+47.3
SUCTION (PSF)	-47.3	-55.4	-55.4	-51.4	-63.4

- EXTERIOR GLAZED OPENINGS IN BUILDING SHALL COMPLY WITH FBC 2017 SECTION 1608 BY EITHER BEING DESIGNATED FOR IMPACT RESISTANCE OR BEING PROTECTED BY IMPACT PROTECTIVE SYSTEMS.
- CORNER DISTANCE, = 3.0 FEET, ASSUMED TRIBUTARY AREA = 10 S.F.



CONCRETE COVER SCHEDULE

CONCRETE CAST AGAINST AND PERMANENTLY EXPOSED TO EARTH:	3"
CONCRETE EXPOSED TO EARTH OR WEATHER:	2"
#6 OR LARGER	1 1/2"
#5 OR SMALLER	
CONCRETE NOT EXPOSED TO WEATHER OR IN CONTACT WITH GROUND:	3/4"
SLABS, WALLS, JOISTS (#11 AND SMALLER)	1 1/2"
BEAMS, COLUMNS (PRIMARY REINF., TIES, STIRRUPS, SPIRALS)	

CONNECTOR SCHEDULE

DESIGNATION	CONNECTOR TYPE	CAPACITY				ACTUAL			REMARKS
		UPLIFT	LATERAL		UPLIFT (MAX)	LATERAL			
			F1	F2		F1 (MAX)	F2 (MAX)		
A	SIMPSON HTS 16	1,310#	415#	1100#	1000#	74#	74#	@ EACH TRUSS, EACH SIDE OF BEAM	
B	SIMPSON CC 66	5,545#	415#	1100#	1000#	74#	74#	@ EACH COLUMN CAP	
C	SIMPSON MPB 66 Z	5,545#	415#	1100#	1000#	74#	74#	@ EACH COLUMN BASE	

NOTE: ALL CONNECTORS / NAILS SHALL BE HOT DIPPED GALVANIZED

FOOTING SCHEDULE

PLAN DESIGNATION	MARK	SIZE			REINFORCING	REMARKS
		WIDTH	DEPTH	LENGTH		
F.60	F.60	6'-0"	16"	6'-0"	(7) #5 EACH WAY @ BOTTOM & (8) #4 VERTICAL BARS WITH 4" HOOK @ TOP AND 9" HOOK @ BOTTOM	MASONRY COLUMN FOUNDATION

STRUCTURAL NOTES